EFFECTS OF GEOMETRY AND MATERIAL ON THE ENERGY DISSIPATION RATE

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ABSTRACT

The concept of energy dissipation rate together with a numerical model which allows for splitting into global plastic work and local work of separation is applied to different specimens and materials, which reveal an extremely different resistance behaviour against ductile crack growth, namely side-grooved C(T) and M(T) specimens of a ferritic steel and thin centre cracked panels with varying crack lengths of an aluminium alloy. In both cases, however, it could be shown, that the dissipated work is mainly due to global plastic deformation of the specimens, once again proving that \( J \) -curves are not an appropriate measure of a material’s fracture toughness. Though the dissipation rate is also geometry dependent, of course, scaling by limit load factors of the respective geometries is successful in certain cases which opens perspectives for transferring experimental data from one geometry to another. The applied cohesive zone model is not only an effective tool for simulating crack growth phenomena but provides a method of characterizing the materials resistance against ductile crack extension by two parameters, namely the cohesive strength, \( \sigma_{\text{max}} \), and the separation energy, \( \Gamma_c \).

INTRODUCTION

The concept of energy dissipation rate, \( R = \frac{dU_{\text{dis}}}{da} \), as proposed by Turner [1] has brought some better understanding of ductile tearing resistance. The rate quantity \( R \), which measures the increment of irreversible external work necessary to propagate the crack by some amount, \( \Delta a \), is physically more meaningful for describing the resistance of a structure against ductile crack extension than the conventionally used cumulative \( J \) integral. Whereas \( J \) -curves keep rising even for steady state crack extension, \( R \) was shown to decrease with \( \Delta a \) and approach a stationary value [2]. Nevertheless, the new concept has not yet found wide acceptance in application, since \( R \), like \( J \), depends on the geometry and the type of loading of the specimen or the structure. Hence, it does not solve the problem of missing transferability of resistance curves. It has been shown for a ferritic steel and two specimen types, namely a C(T) and a M(T) specimen, that this geometry dependence could be scaled by their respective limit load factors [3]. However, a detailed study on the geometry effects on \( R(\Delta a) \) curves for various materials and specimen geometries [4] exhibited limitations of this scaling.

Classical elastic-plastic fracture mechanics suffers from the inherent inability of separating local work of fracture from global work of (remote) plastic deformation. Numerical simulations however make it possible to split up the total dissipated energy [5], within the limitations of any model, of course. This is illustrated on numerical simulations of experimental tests.
DEFINITION AND EVALUATION OF R

Since Griffith's considerations on rupture in solids the "energy approach" to fracture phenomena has become one of two supporting legs of fracture mechanics. It is based on the material independent law of conservation of energy which under quasistatic conditions, is written down for an incremental process between times $t$ and $t+\Delta t$ involving a crack extension of area $\Delta A = \Delta \Delta t$ as

$$ W_{ex} = U_{el} + U_{pl} + U_{sep}, $$

where $W_{ex}$ is the work done by external forces, $U_{el}$ and $U_{pl}$ are the elastic and plastic part of the deformation energy, and $U_{sep}$ is the "work of separation" in the process zone which is necessary to create new surfaces.

The principal difficulty in elastic-plastic fracture mechanics consists in separation of the two dissipative terms in the balance of power, namely the rates of plastic work and fracture energy. Such separation would be necessary in order to formulate a relevant fracture criterion, since $U_{pl}$ is not a material constant but depends on geometry and loading conditions. In the R-curve method, the difference between the two terms is not recognized leading to a number of inconsistencies well known as "constraint effects".

Kolednik [6] gave obvious examples what $J-\Delta a$ curves really mean in gross plasticity. Many attempts have been made to split the dissipated energy into (local) fracture energy and (global) plastic energy, but did not yet yield satisfactory results. Turner [1] doubted that splitting dissipation into fracture and plasticity is possible at all and suggested the combined plastic plus fracture dissipation rate

$$ R = \frac{dU_{dis}}{dA} = \frac{dU_{pl}}{dA} + \frac{dU_{sep}}{dA} = \frac{d(W_{ex} - U_{el})}{dA} $$

(2)

to be fundamental to plastic tearing.

However, Barenblatt's 40 years old idea [7] of a cohesive zone at the crack tip is the key to a model which allows for splitting the total dissipated energy into local and global contributions [4]. Its application has suffered from the fact that the traction-separation law, $\sigma_n - \delta_n$, within the cohesive zone cannot be determined experimentally. The increasing potential of numerical simulations has opened a possibility of determining the parameters of a postulated relation from experimental data by an inverse method. The present study adopts a formulation of Needleman [8]

$$ \sigma_n = \sigma_{max} e^{z \frac{\delta_n}{\delta_c}} \exp \left( -z \frac{\delta_n}{\delta_c} \right), $$

(3)

where $e = \exp (1)$ and $z = 16 e / 9$. Eqn 3 involves two material parameters, the cohesive strength, $\sigma_{max}$, and the cohesive length, $\delta_c$, or alternatively the separation energy, $\Gamma_c$.

$$ \Gamma_c = \frac{9}{16} \sigma_{max} \delta_c. $$

(4)

The two contributions to the dissipation rate, Eqn. 2, can be calculated within a finite element analysis as

$$ U_{pl} = \int \sigma_n \delta_n dV \quad \text{and} \quad U_{sep} = \int_{\delta_n=0}^{\delta_c} \sigma_n d\delta_n \delta_c = \Gamma_c $$

(6)
respectively, assuming that material damage and final separation occurs in the cohesive zone, only, which is embedded in an elastic-plastic continuums, see Figure 1. The cohesive zone is modelled by special user defined elements in the FE code ABAQUS.

![Finite element mesh with cohesive elements in the ligament](image)

**Figure 1:** Finite element mesh with cohesive elements in the ligament

## RESULTS

The above model was applied to simulate crack growth in
- a side-grooved M(T), $2W = 100$ mm, $a_0/W = 0.49$, and a side-grooved C(T) specimen, $W = 50$ mm, $a_0/W = 0.59$, respectively, of StE 460, both under conditions of plane strain, see [9, 10], and
- thin centre cracked panels, $2W = 508$ mm, $B = 1.0$ mm, $a_0/W = 0.2$, $0.35$, $0.55$, of Al 2024 T3, under condition of plane stress, see [11].

All the simulations are based on experimental tests [12, 13]. Ductile crack initiation occurred under fully plastic conditions in the ferritic steel, and under contained yielding in the aluminium sheets. The cohesive parameters used for the numerical simulations were
- $\sigma_{\text{max}} = 3.36 \sigma_Y = 1579$ MPa and $\Gamma_c = 53.3$ N/mm for the side-grooved steel specimens, and
- $\sigma_{\text{max}} = 2 \sigma_Y = 570$ MPa and $\Gamma_c = 17$ N/mm for the thin aluminum sheets.

As has been shown in [9], these parameters will depend on the stress triaxiality, in general, which especially becomes evident from the $\sigma_{\text{max}}$ values for the plane strain and the plane stress case, respectively.

![Graphs](image)

**Figure 2:** C(T) and M(T) specimens of StE 460, (a) $J_R$-curves from fracture tests and numerical simulations, (b) rate of plastic work normalized by work of separation
Figure 2a shows a comparison of test results [12] with numerical simulations [10] for the M(T) and the C(T) specimen of StE 460 confirming the good performance of the model. The numerically calculated global plastic work per crack increment is shown in Figure 2b. It decreases with crack growth and approaches a stationary value which is around five times higher for the M(T) than for the C(T). And it is by a factor of 10 to 30 for the C(T) and 50 to 150 for the M(T) greater than the local work of separation, which means that for the ferritic steel "fracture resistance", as measured by an R-curve test, is mainly due to global plastic deformation of the specimen or structure. This is confirmed by Figure 3a where a plastic $J$ value is calculated from the accumulated plastic work by

$$J_{pl} = \eta \int_0^{\Delta a} \frac{dU_{pl}}{da} \, da + J_0$$

with

$$\eta = \begin{cases} 2.26 & \text{for C(T)} \\ 1.0 & \text{for M(T)} \end{cases}$$

and compared to the experimental (total) $J$. This explains the geometry dependence of $J_R$-curves as well as the scaling properties of the limit load factor,

$$R_n = \sigma_y \left( W - a_0 \right) \frac{\sigma_y}{E} \cdot f_Y$$

with

$$f_Y = \begin{cases} \frac{2}{\sqrt{3}} & \text{for C(T)} \\ 1 - \frac{a_0}{W} & \text{for M(T)} \end{cases}$$

see [13] and Figure 3b, where the rate of plastic work is normalized by $R_n$, bringing the curves of C(T) and M(T) together and thus render transferability.

**Figure 3:** C(T) and M(T) specimens of StE 460,
(a) $J_R$-curves from tests and from accumulated plastic work Eqn 7,
(b) rate of plastic work normalized by limit load factor Eqn 3.
Figure 4a displays a comparison of test results [13] with numerical simulations [11] for the thin aluminium centre cracked panels. The numerically calculated dissipated work, i.e. work of global plastic deformation plus local separation, is plotted in Figure 2b increasing almost linearly with crack growth and depending on the initial crack lengths, of course.

**Figure 4:** Centre cracked panels of Al 2024 T3,  
(a) tests and numerical results of applied stress vs crack growth,  
(b) accumulated plastic work from FE simulation.

As ductile crack initiation occurs under contained yielding the dependence of the dissipation rate on crack extension looks much different, see Figure 5a, from that for the ferritic steel, see Figure 2b. It does not decrease monotonically as in Figure 3b but displays a maximum after 20 to 30 mm of crack growth which
corresponds approximately to the load maximum in Fig 4a and presumably with reaching a fully yielded condition. Normalization by the local work of separation, $\Gamma_c$, shows again that a great part of the dissipated work is due to plastic deformation as $R$ is 20 to 60 times higher than $\Gamma_c$, see Figure 5a. Normalization by the plastic limit load factor of Eqn 3 with $f_Y = 1$ for plane stress has some scaling effect on the dissipation rates for the three crack lengths and brings the three curves closer together, see Figure 5b.

**CONCLUSIONS**

The dissipation rate, $R$, characterizes the resistance of a structure against ductile crack propagation. It is, hence, geometry dependent, as it includes the total work of plastic deformation per crack increment. Numerical modelling helps to overcome the fundamental deficiency of classical elastic-plastic fracture mechanics, i.e. its inability of separating local work of fracture from work of remote plasticity. This splitting is realized by introducing a cohesive zone in the crack ligament where material separation is supposed to be localized. The model was successfully applied to various materials and specimens which had been chosen since they show an extremely different crack resistance behaviour.

It was found that

- a great part of the dissipated work is due to global plastic deformation, explaining the geometry dependence of $J_R$-curves which represent accumulated dissipated work,
- normalization of $R$-$\Delta a$-curves by a plastic limit load factor has some scaling effect with respect to a geometry independent description of crack growth resistance.

The applied cohesive zone model is not only an effective tool for simulating crack growth phenomena but provides a method of characterizing the materials resistance against ductile crack extension by two parameters, namely the cohesive strength, $\sigma_{\text{max}}$, and the separation energy, $\Gamma_c$.

**REFERENCES**